From:

## FHWA Geotechnical Aspects of Pavements Reference Manual

## Chapter 5.0 Geotechnical Inputs For Pavement Design (continued) 5.4 Mechanical Properties

Strength or Index Property	Correlation	Comments	Test Standard
California Bearing Ratio <sup>2</sup>	Mr (psi) = 2555 (CBR) <sup>0.64</sup> Mr (MPa) = 17.6 (CBR) <sup>0.64</sup>	CBR = California Bearing Ratio (%)	ASTM D1883, AASHTO T193:The California Bearing Ratio
Stabilometer R-value	Mr (psi) = 1155 + 555 R Mr (MPa) = 8.0 + 3.8 R	R = R-value	AASHTO T190-Resistance R-Value and Expansion Pressure of Compacted Soils
AASHTO layer coefficient	$\begin{array}{l} Mr \; (psi) = 30,000 \; {(a_i \; / \; 0.14)}^3 \\ Mr \; (MPa) = 207 \; {(a_i \; / \; 0.14)}^3 \end{array}$	a <sub>i</sub> = AASHTO layer coefficient	AASHTO Guide for the Design of Pavement Structures (1993)
Plasticity index and gradation	CBR =	w PI = P200 * PI P200 = % passing No. 200 sieve size PI = plasticity index (%)	AASHTO T27-Sieve Analysis of Coarse and Fine Aggregates AASHTO T90-Determining the Plastic Limit and Plasticity Index of Soils
Dynamic Cone Penetration <sup>3</sup>	CBR = 292 / (DCP <sup>1.12</sup> )	CBR = California Bearing Ratio (%) DCP = Penetration index, in./blow	ASTM D6951-Standard Test Method for Use of the Dynamic Cone Penetrometer in Shallow Pavement Applications

## Correlations<sup>1</sup> between resilient modulus (Mr) and various material strength and index properties (NCHRP 1-37A, 2004).

1 Correlations should be applied to similar conditions - i.e., CBR measured at optimum moisture and density vs. soaked conditions correlates to Mr at corresponding moisture and density conditions.

2 NCHRP 1-37A strongly recommends against use of the older Heukelom and Klomp (1962) correlation Eq. (5.13) between Mr and CBR specified in the 1993 AASHTO Design Guide.

3 Estimates of CBR are used to estimate Mr.

http://www.fhwa.dot.gov/engineering/geotech/pubs/05037/05b.cfm